

USING OFFSHORE SAMPLE QUALITY METHODOLOGY FOR ONSHORE INVESTIGATIONS

Tom Lunne, NGI

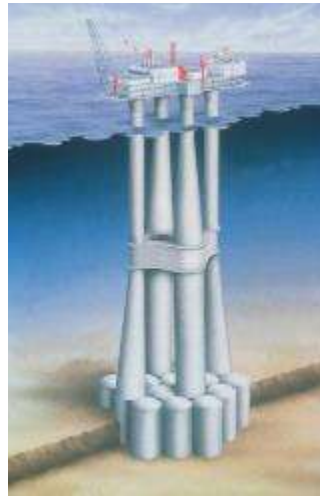
USING OFFSHORE SAMPLE QUALITY METHODOLOGY FOR ONSHORE INVESTIGATIONS

- Background
- Development of sample quality criteria for marine clays
- Application for onshore projects in Norway
- Application for UK clays
- Need for other method(s) in overconsolidated clays
- Summary and conclusions

Background

- Sample quality is as important onshore as offshore
- In connection with deep water oil and gas development offshore Norway, sample quality became key issue
- This led to comprehensive R&D work on establishing a criteria for evaluating sample quality
- The sample quality criteria has been accepted in the ISO standard for offshore and also in Norwegian adoption of Eurocode for onshore works

Large gravity platform in Norwegian Trench



Water depth: 320 m

Platform height: 500 m

Skirts: 36 m long

Large forces, including
cyclic

Very soft soils in upper
layers

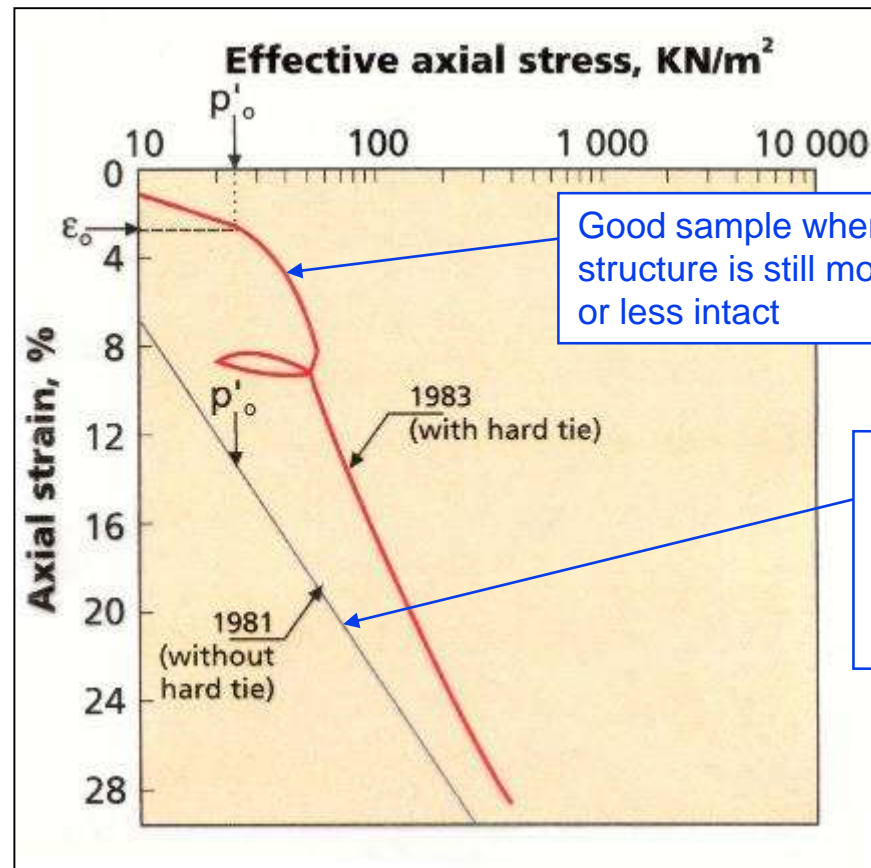
Accurate soil parameters required for installation and long term behaviour

Classical example from Troll field

Soil investigations started in 1981

Primitive method for controlling heave during drilling and sampling – *bad sample quality*

In 1983 a new method for heave compensation was developed – good sample quality

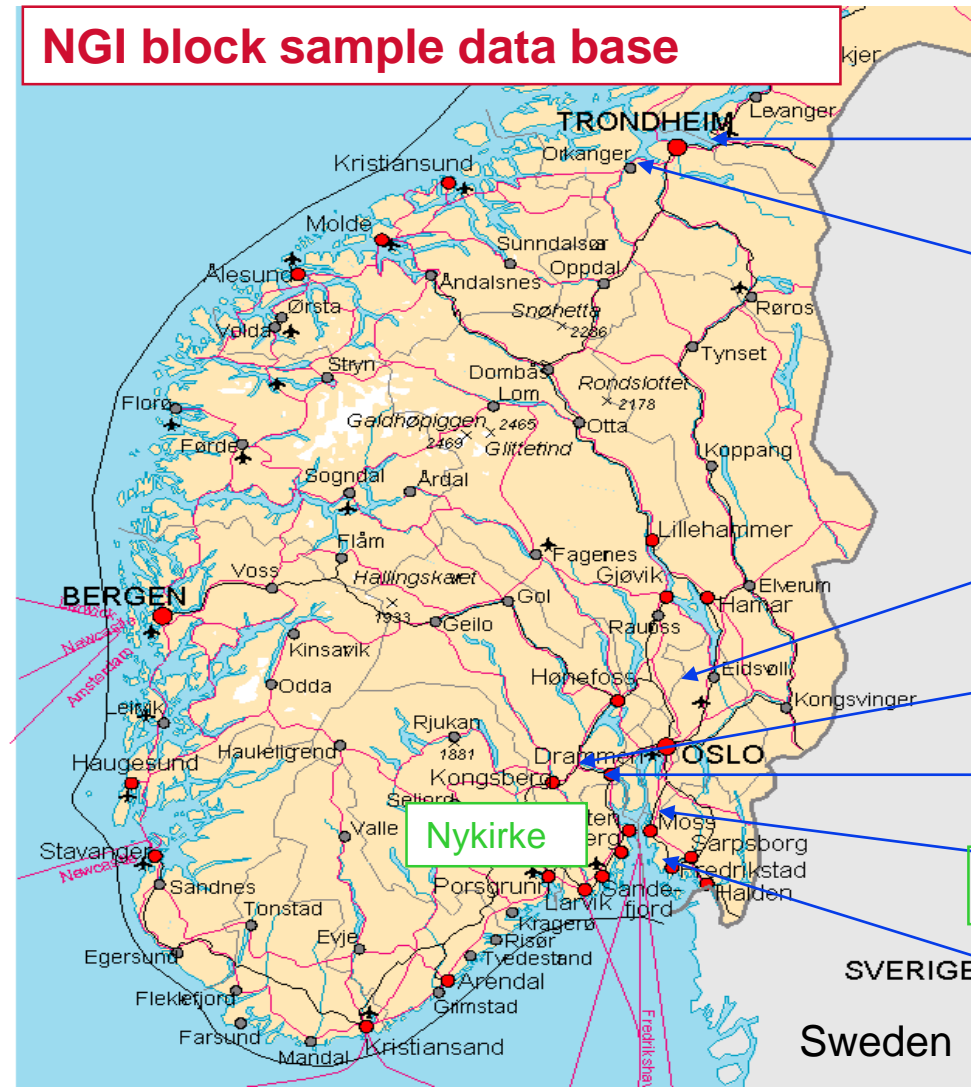


Results of CRSC oedometer test

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NGI block sample data base



Glava

Kvenild

Eidsvold

Lierstranda

Drammen

Emmerstad

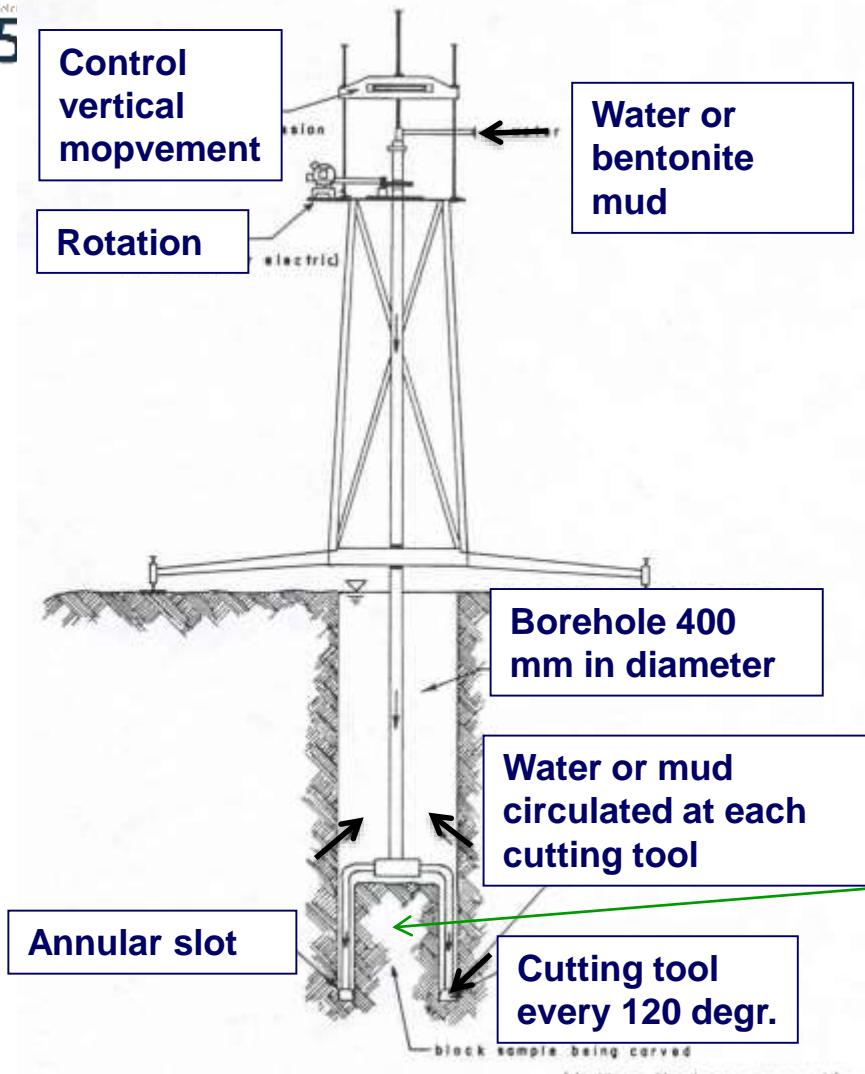
Onsøy

Nykirke

At a number of soft clay sites laboratory tests on high quality and low quality samples were carried out

NGI DATA BASE OF HIGH QUALITY BLOCK SAMPLES, 75 MM AND 54 MM PISTON TUBE SAMPLES

- ***Block sampler*** : the best possible sample today (only onshore – use as benchmark)
- ***75 mm thin wall piston sampler*** : the best possible offshore sample today
- ***54 mm composite (w/liner) piston tube sampler*** : bad sample quality



CANADIAN SHERBROOKE BLOCK SAMPLER

Sample dia= 250 mm
Sample height = 350 mm

Lefebvre and Poulin, 1979

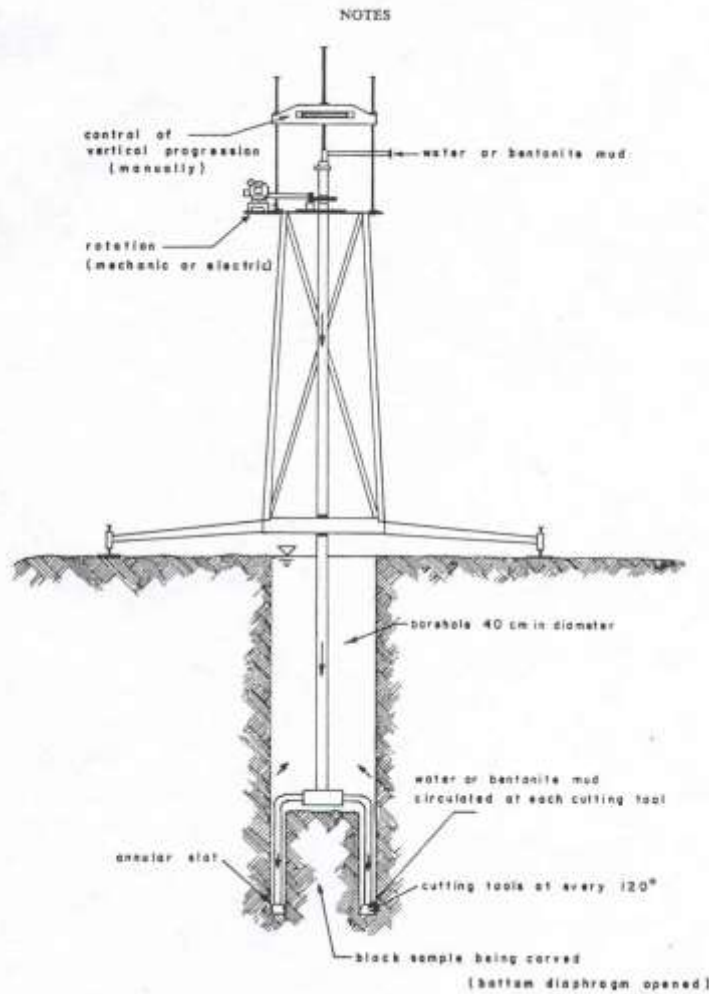


FIG. 6. Schematic view of a block sample being carved.

CANADIAN SHERBROOKE BLOCK SAMPLER



Cutting edge

Sherbrooke block sampler

THE DRILL RIG USE TO OPERATE THE SHERBROOKE BLOCK SAMPLER





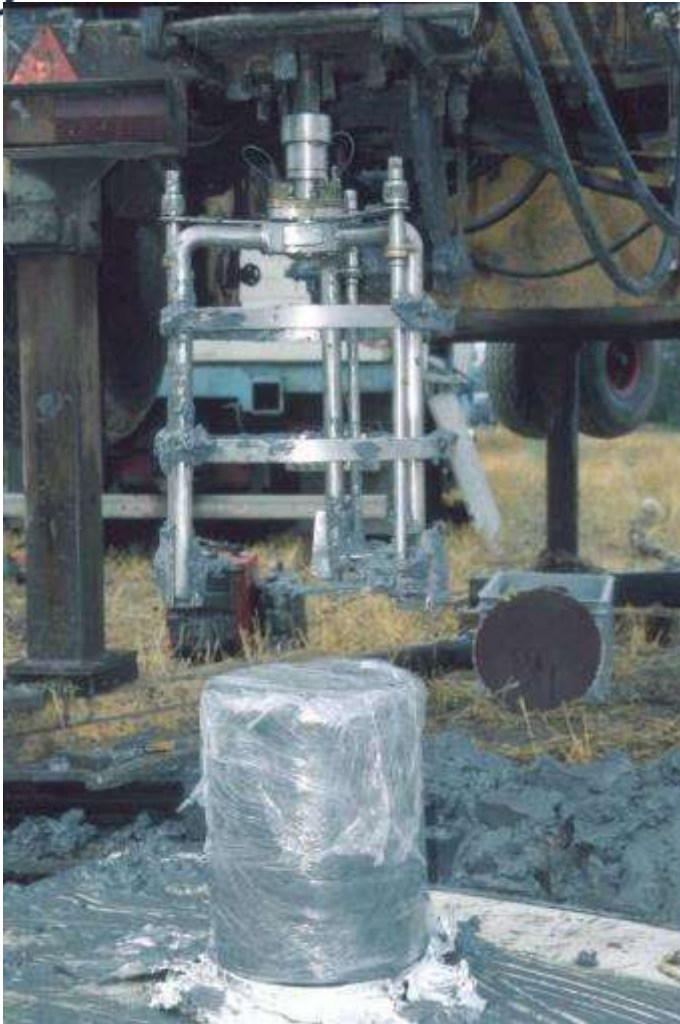
Sampler is lowered into borehole

BLOCK SAMPLING WITH SHERBROOKE SAMPLER

Install casing through upper crust



Sample as recovered



BLOCK SAMPLING WITH SHERBROOKE SAMPLER

Block sample cleaned
and wrapped in plastic
cling film and sent to
laboratory

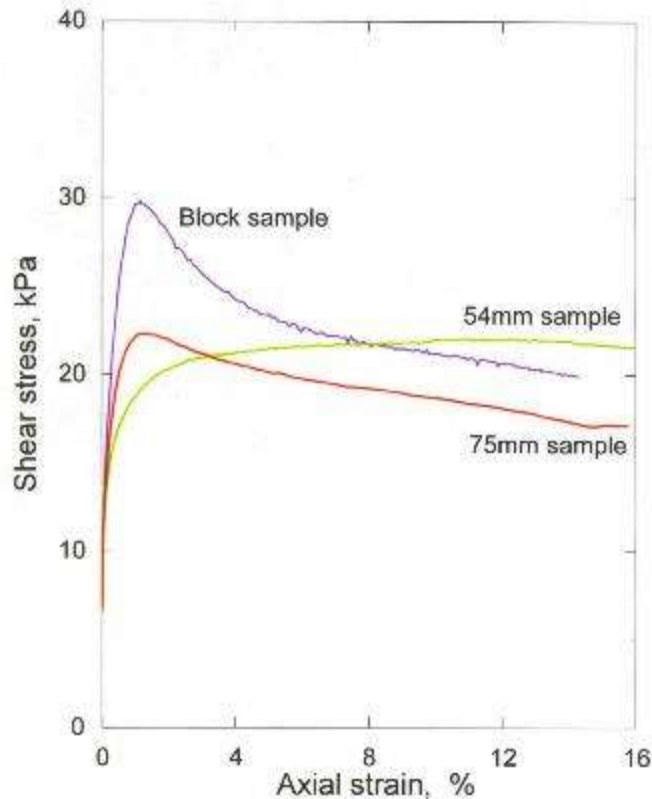
BASIC LABORATORY TESTS



CRS CONSOLIDATION TEST : constant rate of strain

**CAUC/CAUE TRIAXIAL TESTS : Consolidated
anisotropically to best estimate of in situ stresses and
sheared in compression or extension**

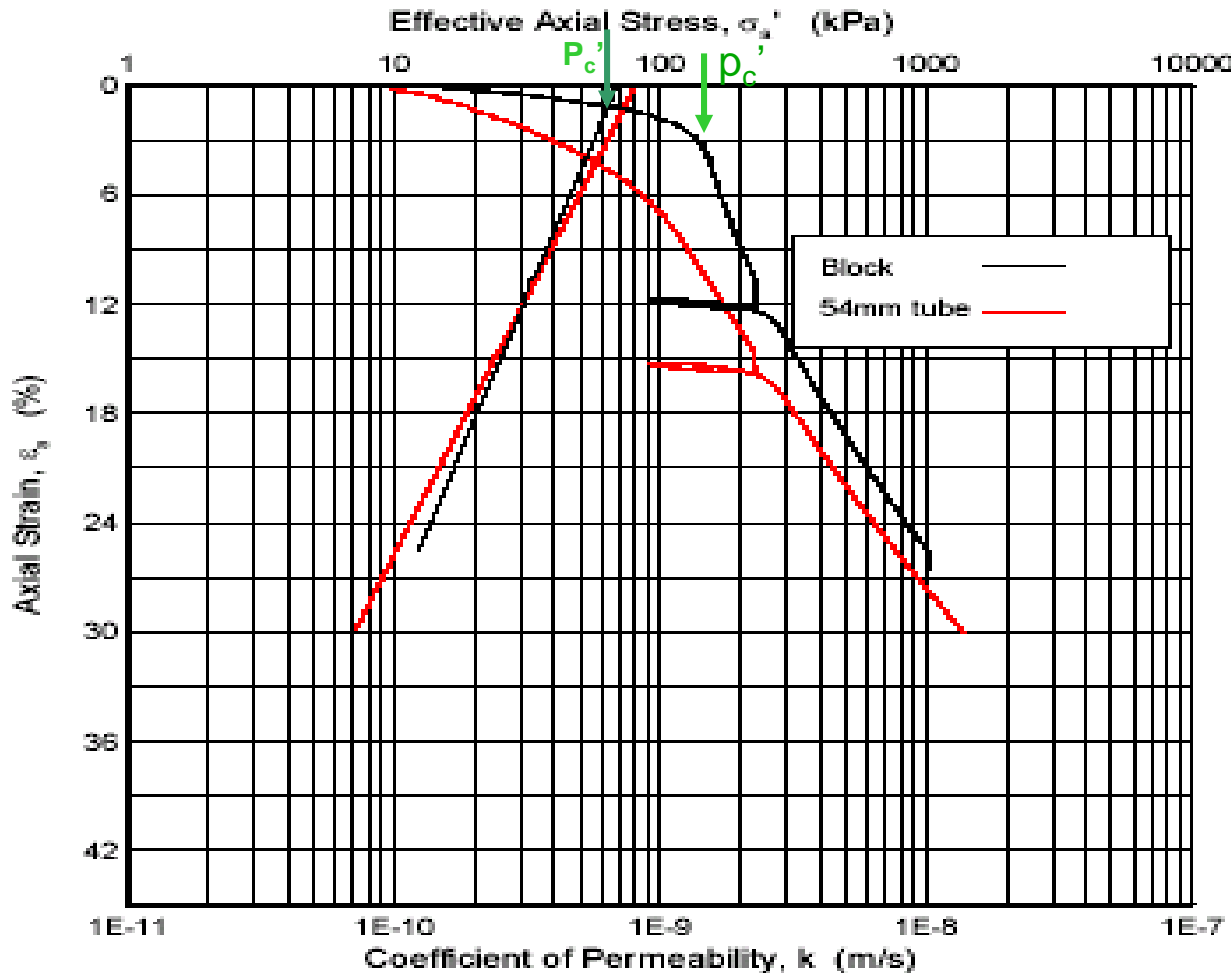
RESULTS FROM SHEARING PHASE OF CAUC TESTS



Shear stress versus axial strain for
CAUC tests at 6.1m depth.

**Lierstranda clay
from 6.1 m depth**

Results of CRSC tests on block and 54 mm samples

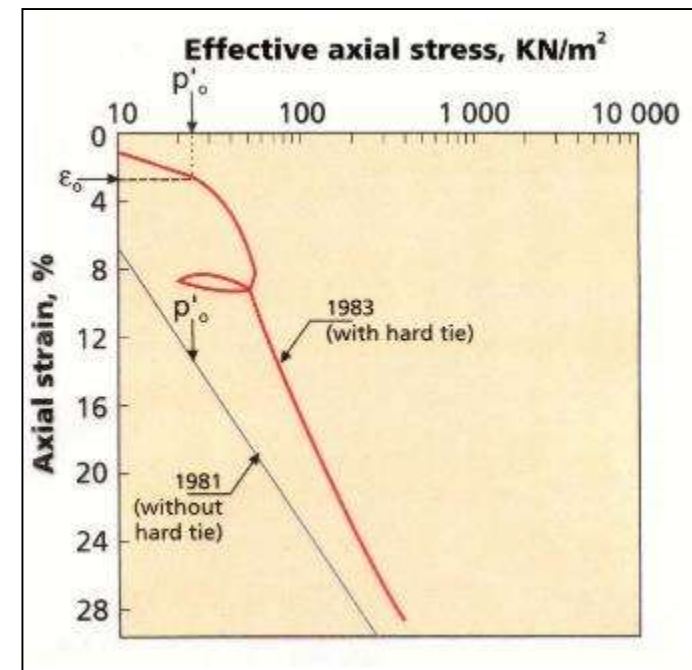


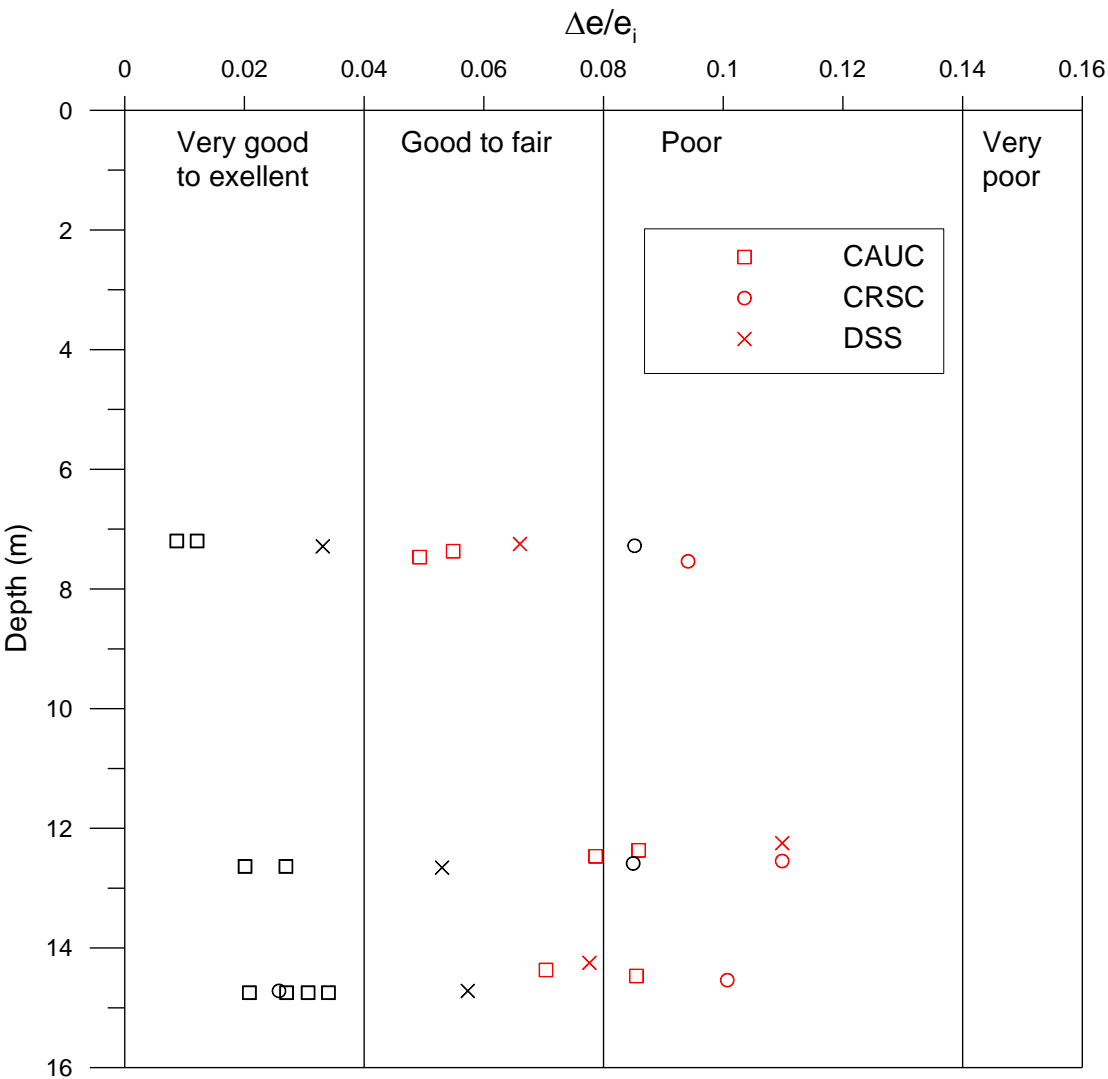
ONSØY CLAY

CRITERIA FOR QUANTIFYING SAMPLE DISTURBANCE

Volume change when consolidating a sample back to in situ stresses expressed as change in void ratio: $\Delta e/e_i$

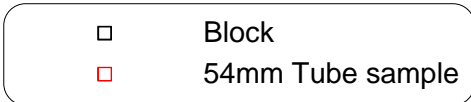
Oedometer test results on good and severely disturbed sample from Troll field illustrating strain (equivalent to $\Delta e/e_i$) as indicator of sample disturbance

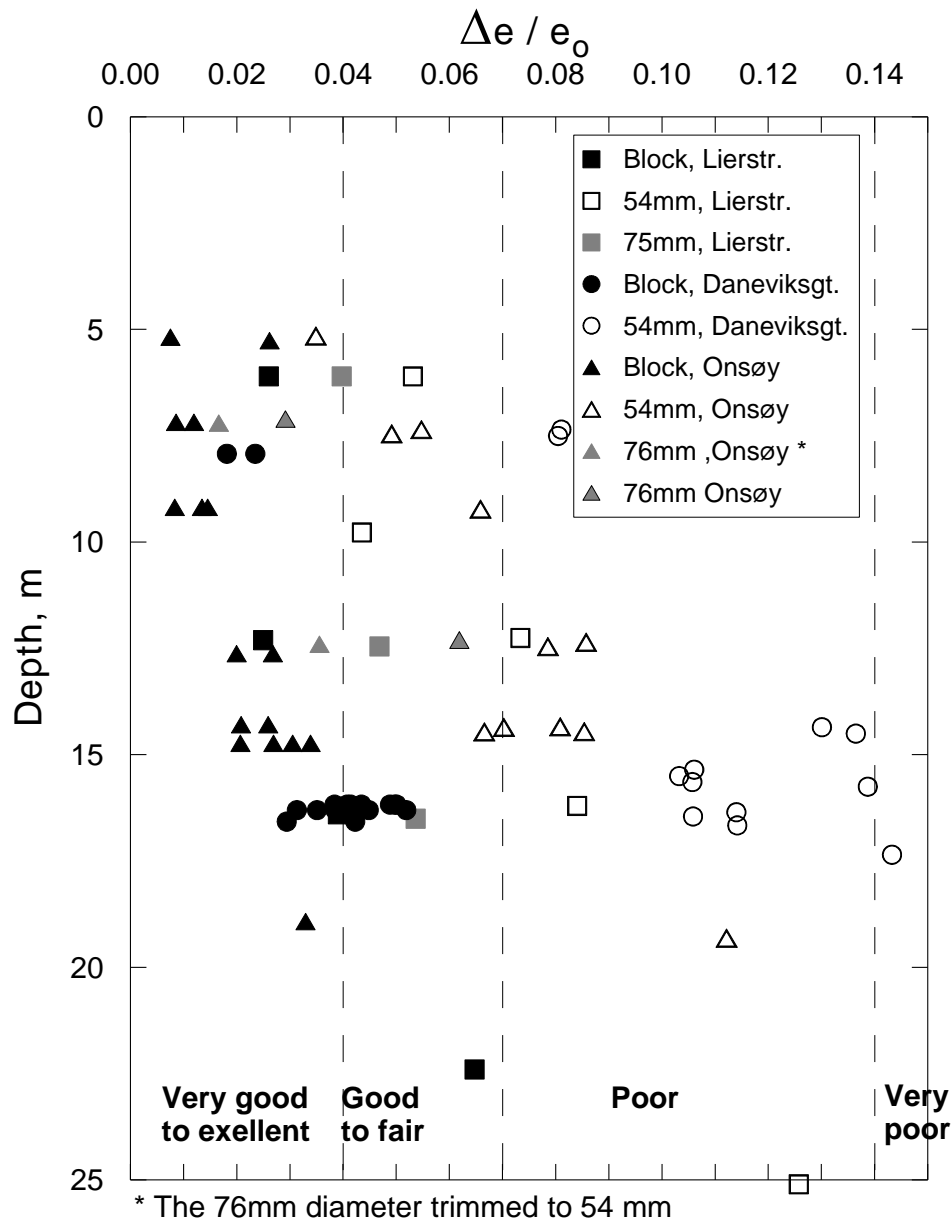




Onsøy clay $\Delta e/e_i$ from CRSC and CAUC tests

Based on similar results from a number of marine clays in Norway and one clay in Scotland (Bothkennar); NGI has developed a set of criteria for evaluation of SD





NGI's scale for quantifying sample disturbance based on change in void ratio when consolidating sample to best estimate of in situ stresses

Based on NGI's data base with block samples and piston samples

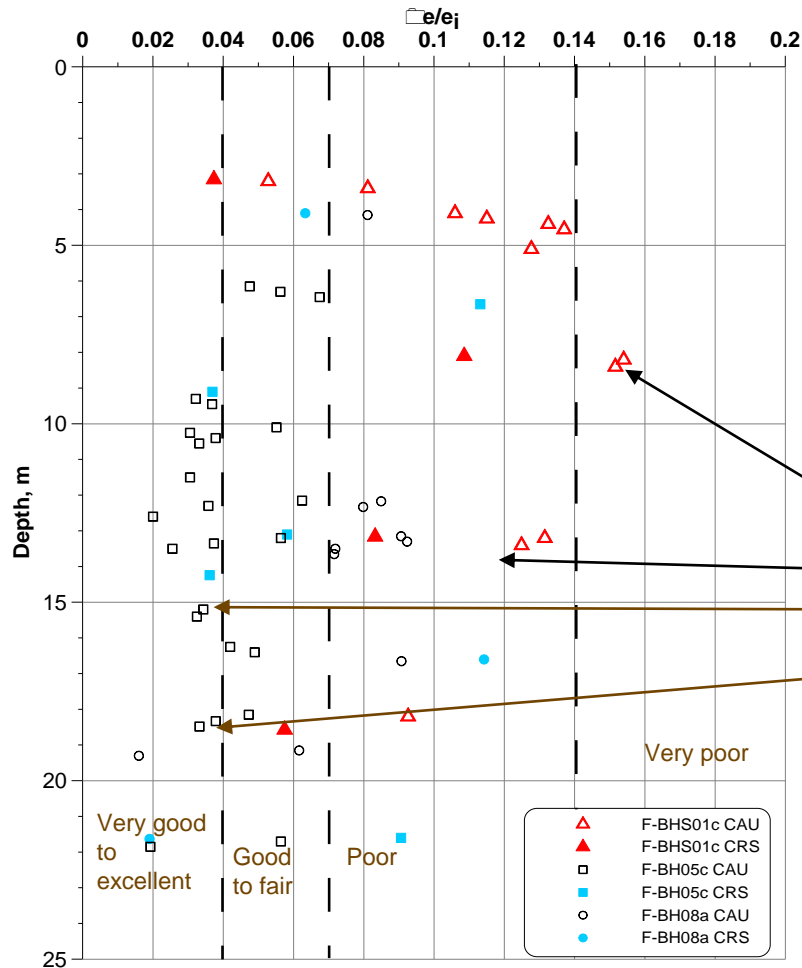
NGI'S CRITERIA FOR SAMPLE DISTURBANCE

Overcon- solidation ratio	$\Delta e/e_0$			
	Very good to excellent*	Good to fair*	Poor*	Very poor*
1 - 2	<0.04	0.04-0.07	0.07-0.14	>0.14
2 - 4	<0.03	0.03-0.05	0.05-0.10	>0.10

Based on CAUC and CRSC tests on Sherbrooke block samples and tube samples in Norwegian soft clays

Has been used at NGI since 1996, SQ evaluation included in all lab reports.

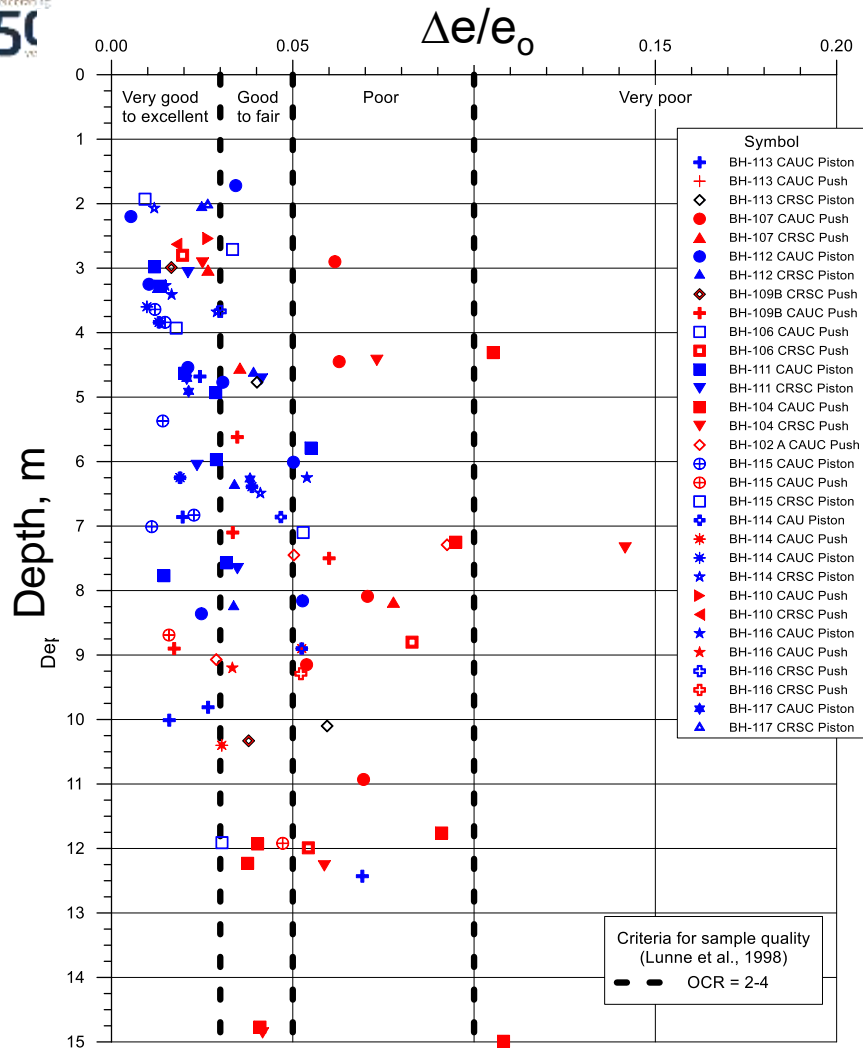
EFFECT OF SAMPLE QUALITY ASSESSMENT OFFSHORE MALAYSIA



Two types of samplers: 1st phase of testing showed thin walled piston samples are of highest quality. Focussed on these in remaining testing programme.

Gravity core samples

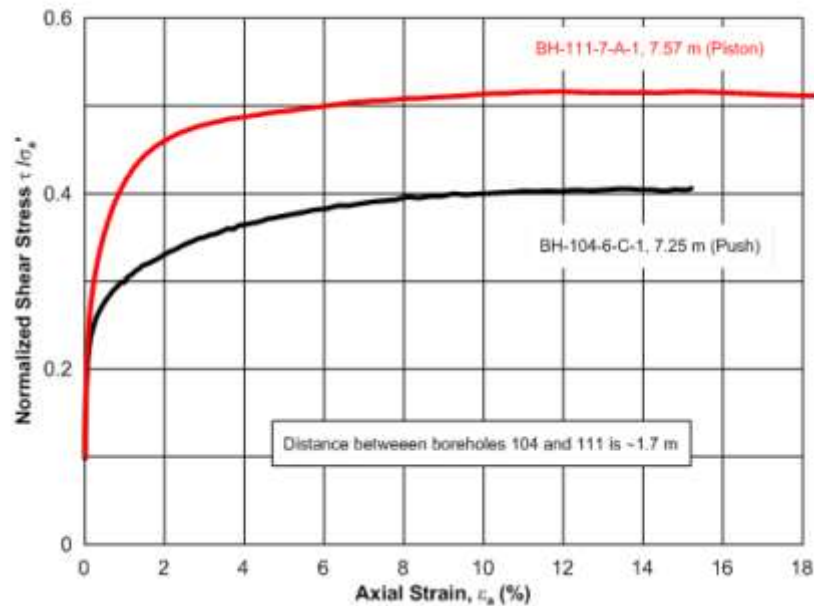
Downhole thin walled piston samples



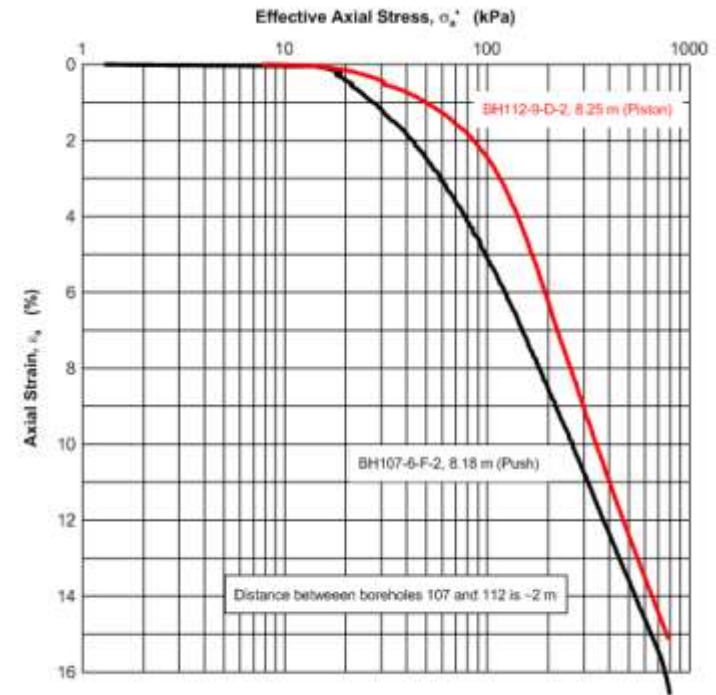
Sample quality evaluation at Barents Sea site

Red is push sampler
Blue is piston sampler

Example downhole sampling in medium stiff clay in Barents sea



Results of CAUC tests



Results of CRSC tests

INTERNATIONAL
STANDARD

ISO
19901-8



First edition
2014-12-01

The Sample
Quality Criteria is
now included in
new ISO standard

**Petroleum and natural gas
industries — Specific requirements
for offshore structures —**

Part 8:
Marine soil investigations

INTERNATIONAL STANDARD

ISO 19901-8



The values of $\Delta e/e_0$ and ε_{vol} should be computed and reported for laboratory consolidation tests conducted on intact clay soils (e.g. incremental load oedometer, constant rate of strain and anisotropic consolidation phase of strength tests such as triaxial and direct simple shear), provided the best estimate *in situ* effective stresses are given. The sample quality is determined using [Table 6](#) for the method of Lunne et al.(2006). An alternative method is given by Terzaghi et al. (1996).

Table 6 — Evaluation of intact sample quality for low to medium OCR clays

OCR	$\Delta e/e_0$ at σ'_{v0}			
	1 to 2	< 0,04	0,04 to 0,07	0,07 to 0,14
2 to 4	< 0,03	0,03 to 0,05	0,05 to 0,10	> 0,10
Sample quality	1 (very good to excellent)	2 (fair to good)	3 (poor)	4 (very poor)

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Application for onshore projects in Norway



The last version of the **Norwegian Geotechnical Society Guideline** (2013) for sampling requires that sample quality shall be documented in soil investigation reports.

Tabell 6. Veiledende kriterier for evaluering av prøve kvalitet basert på endring i porettall.

OCR	$\Delta e/e_0^4$			
	Veldig god til utmerket	God til brukbar	Dårlig	Veldig dårlig
1-2	<0,04	0,04-0,07	0,07-0,14	>0,14
2-4	<0,03	0,03-0,05	0,05-0,10	>0,10

Application for onshore projects in Norway

Some project examples on importance of sample quality

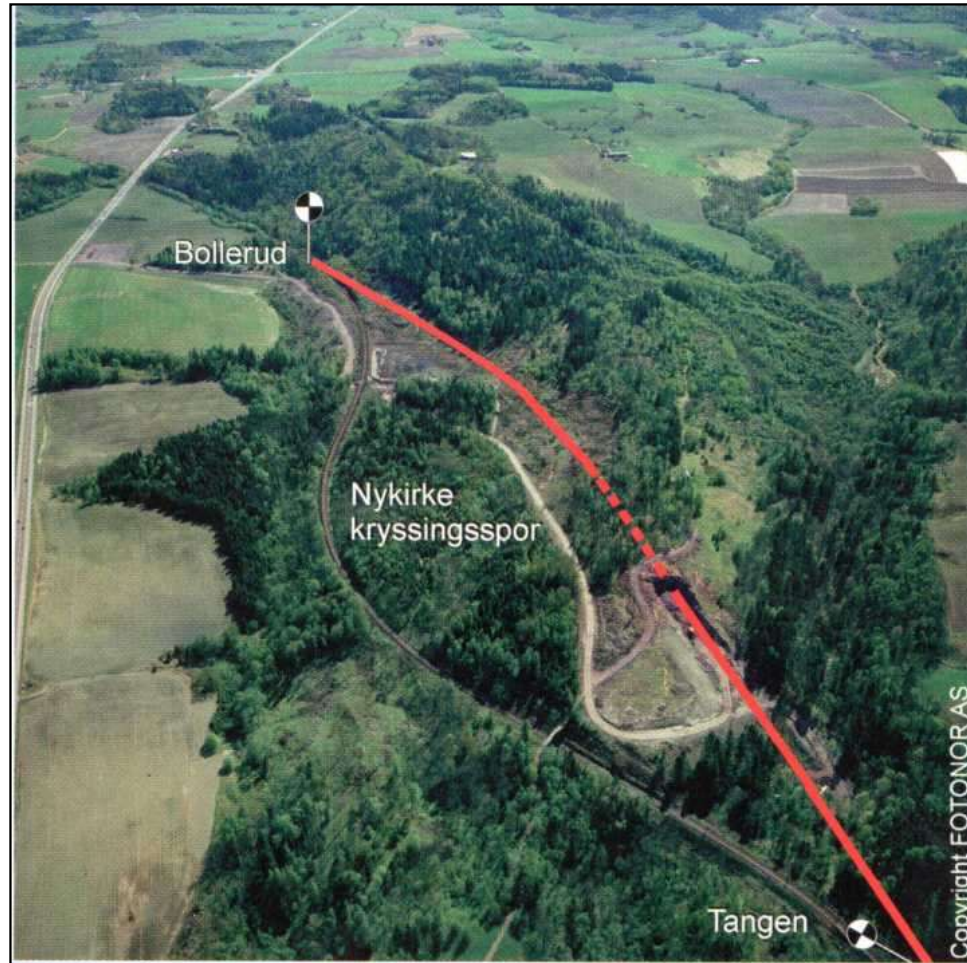
- Fill on soft clay in connection with new railway track south of Oslo
- Very low plasticity clays

Example in connection with upgrading of railway system south of Oslo



Ref. paper by Hermann and Jensen (2000)

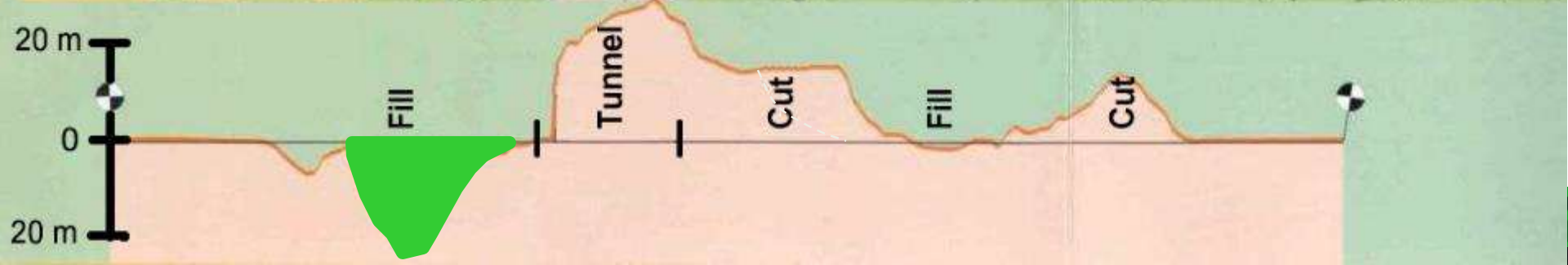
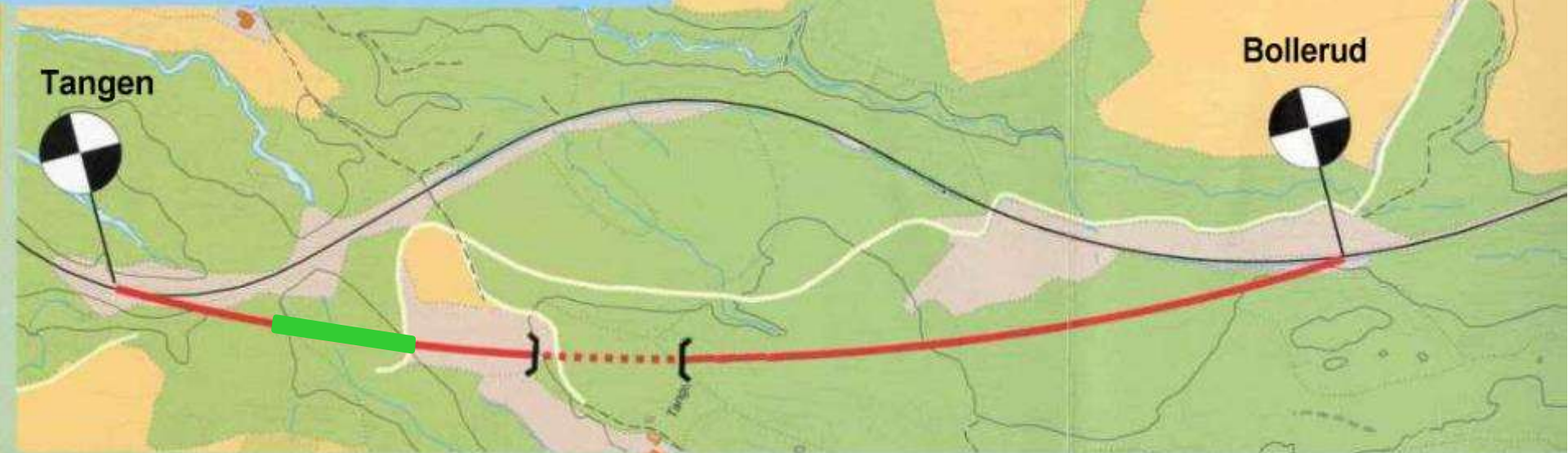
NEW RAILWAY TRACK NYKIRKE, NORWAY

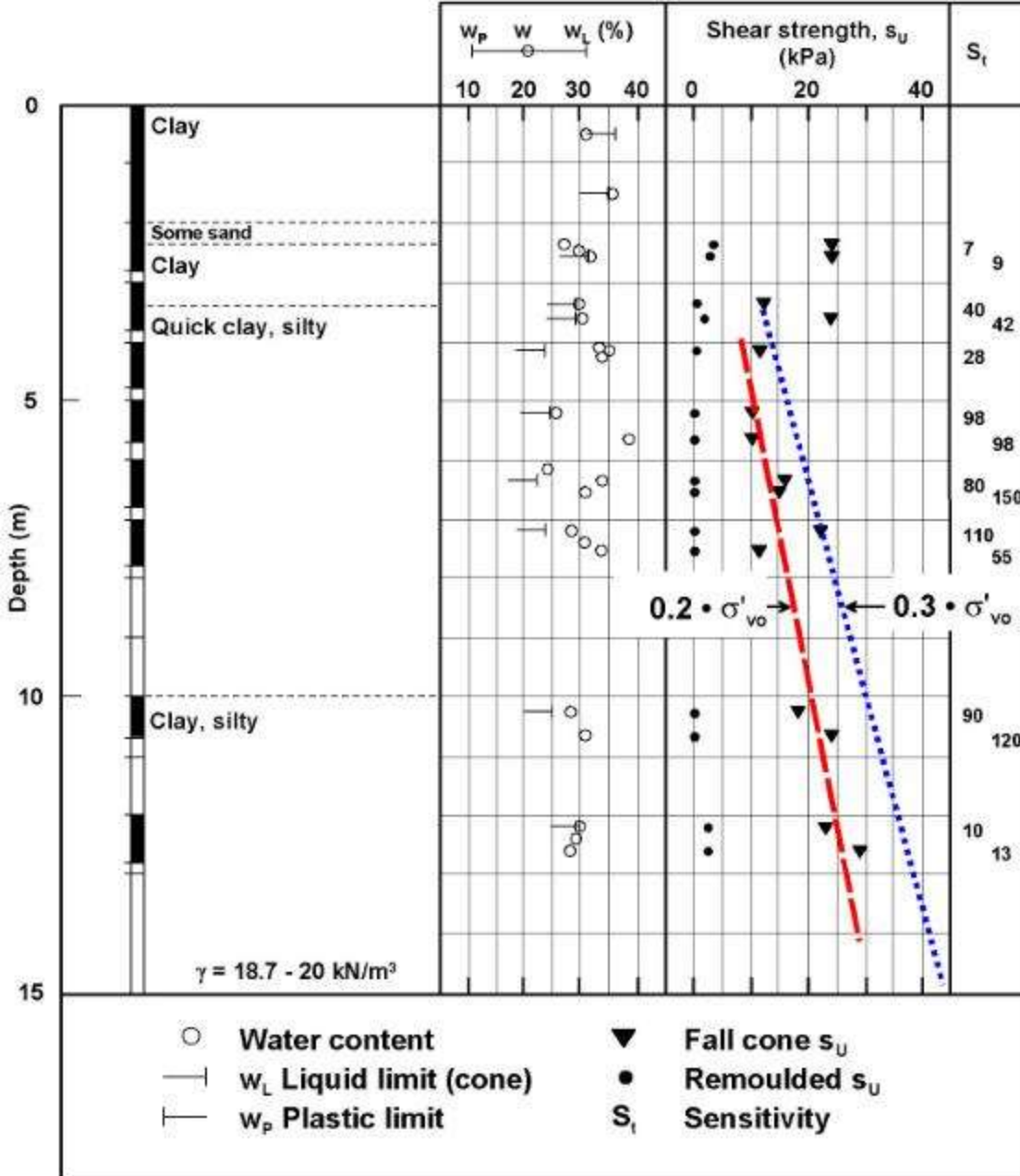


New double track route

Existing single track railway ————
New double track railway ————
Construction road ————

Scale
0 100 200 300 400 m

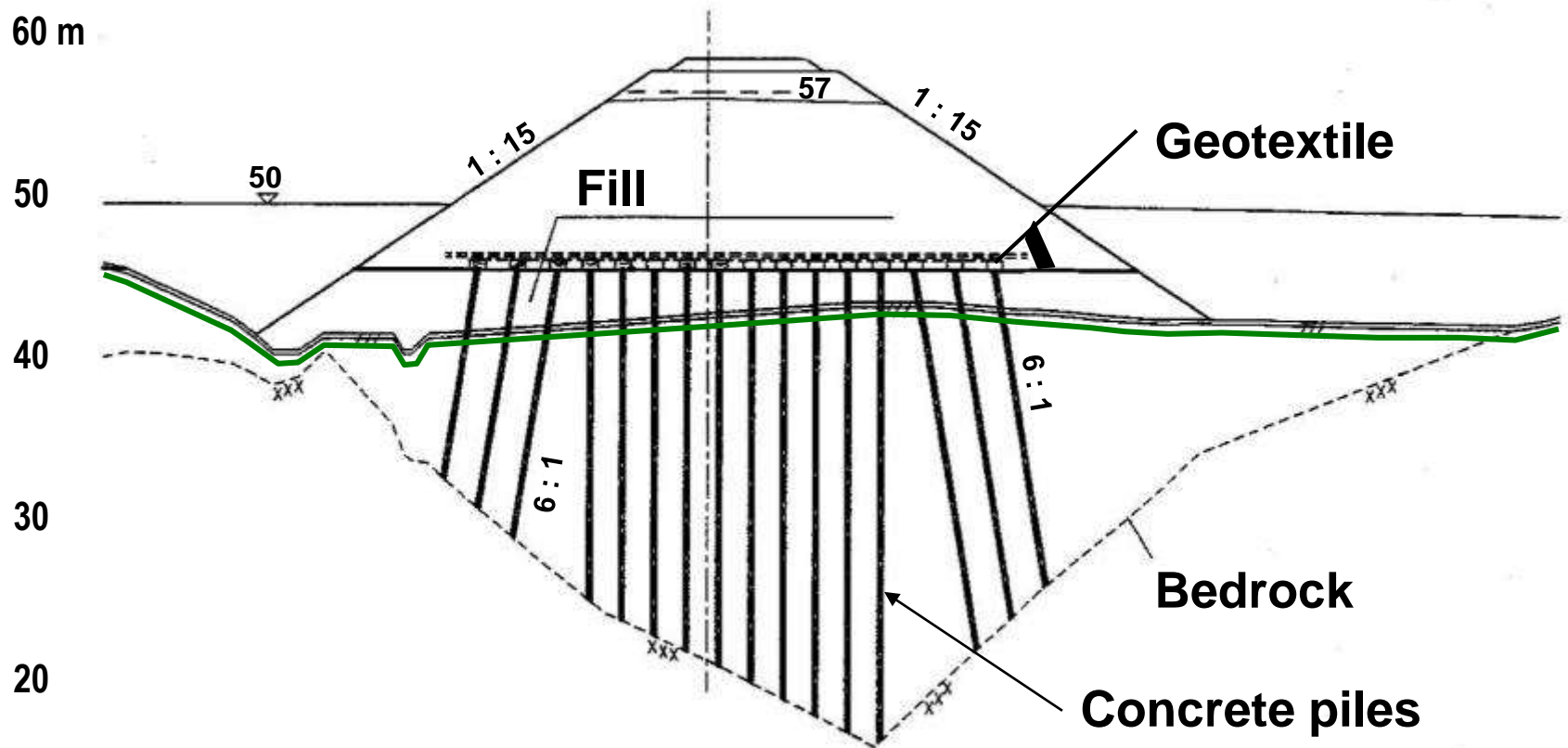




Results of standard soil boring with 54 mm composite piston sample

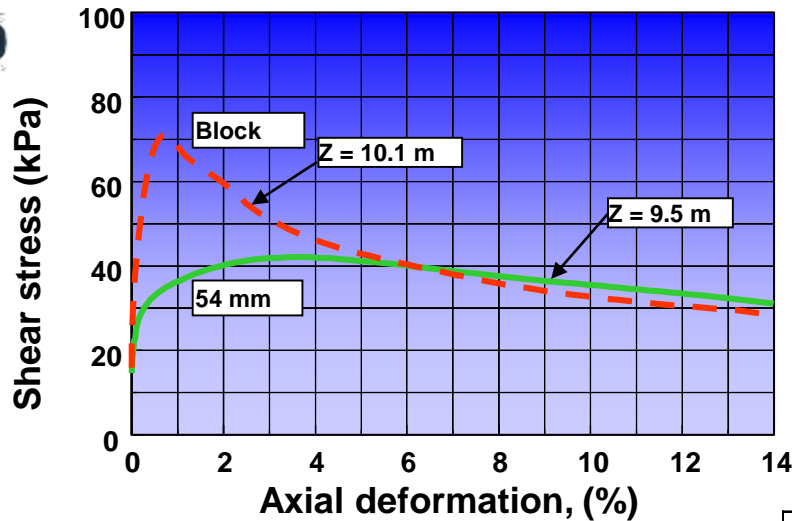
Given in tender documents

Initial solution in tender documents

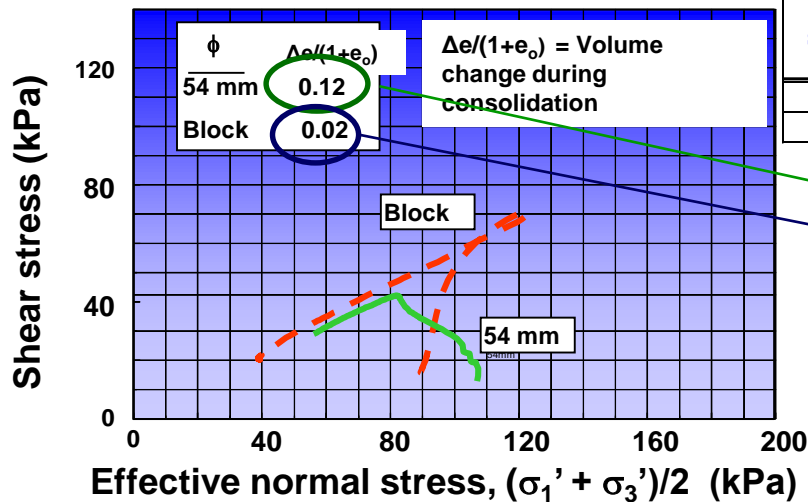


Example in connection with upgrading of railway system south of Oslo

NGI as subcontractor to main contractor evaluated the soil data given by client to be of bad quality and recommended CPTU and high quality block samples be obtained



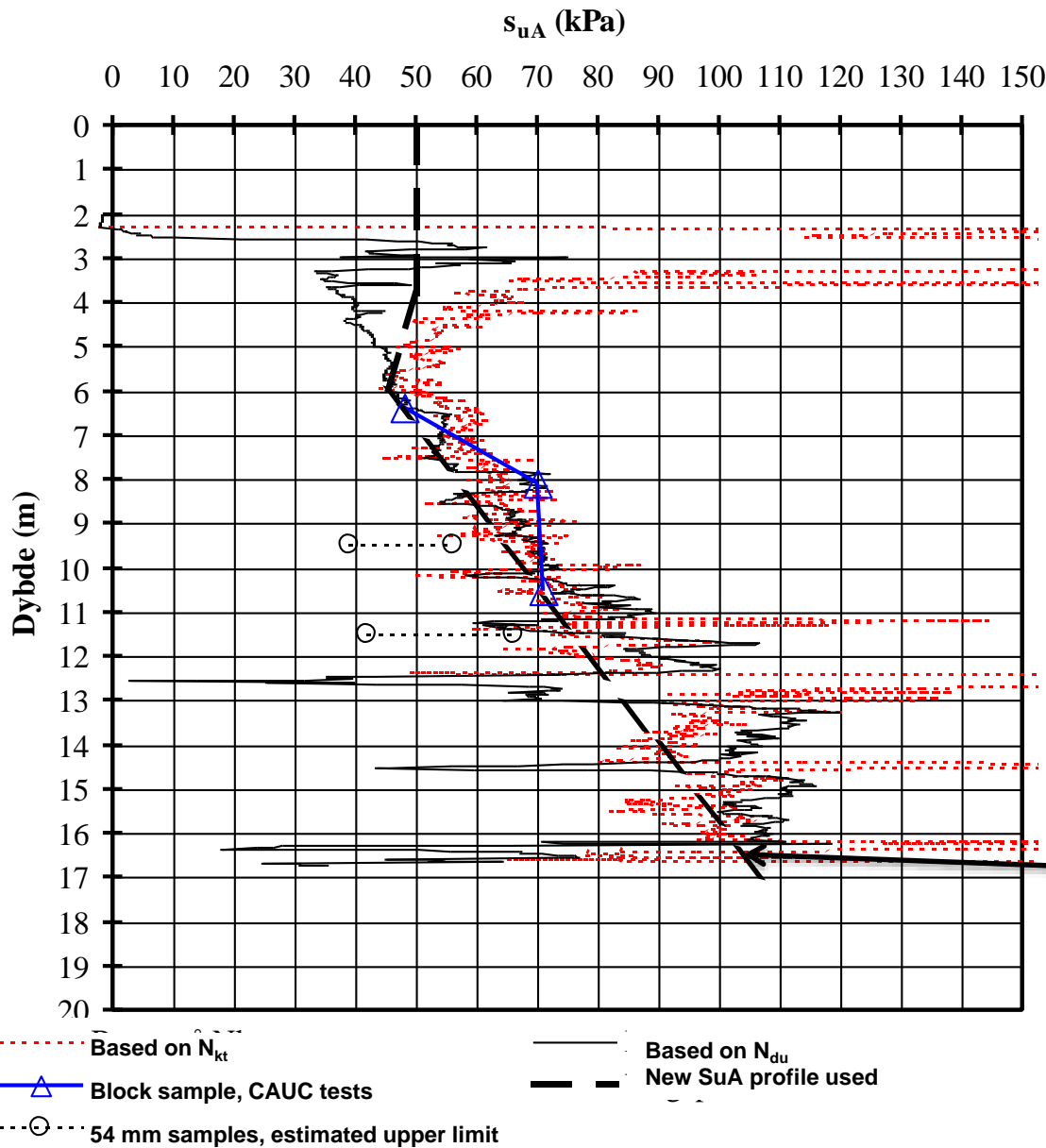
Results of CAUC triaxial tests on new block samples compared with old 54 mm piston samples



NGI scale sample quality

Overconsolidation ratio	$\Delta e/e_0$			
	Very good to excellent	Good to fair	Poor	Very poor
1 - 2	<0.04	0.04-0.07	0.07-0.14	>0.14
2 - 4	<0.03	0.03-0.05	0.05-0.10	>0.10

Both $\Delta e/1+e_0$ values and shape of stress paths confirm much higher quality of block sample



Example in connection with upgrading of railway system south of Oslo

NEW CAUC TESTS CONFIRMED INTERPRETED S_U VALUES FROM CPTUS

Design shear strength for final design

CASE HISTORY NYKIRKE RAILWAY TRACK

**Upgraded shear strength profile resulted
in possible change in technical solution:**

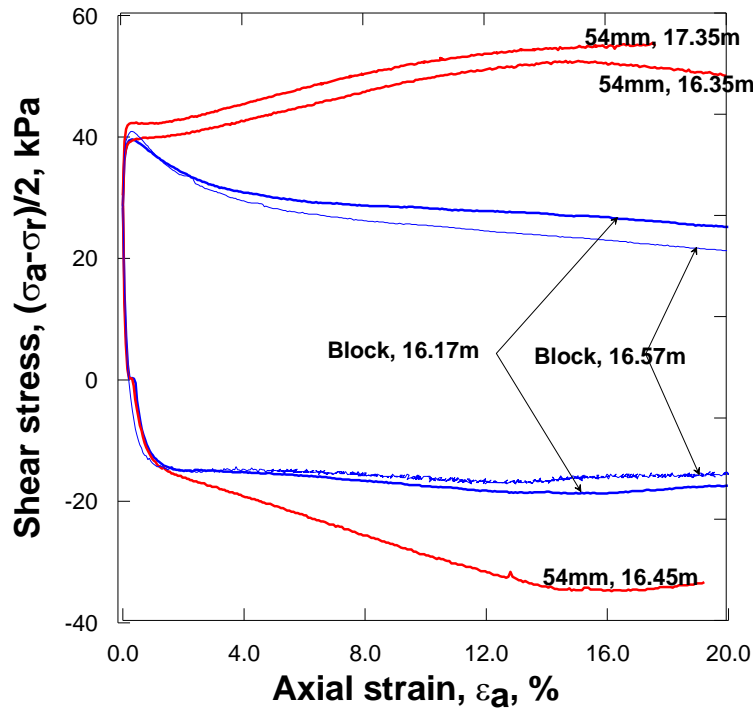
**From stability viewpoint not necessary with piles to
rock**

**Settlements could be taken care of by vertical drainage
combined with preloading**

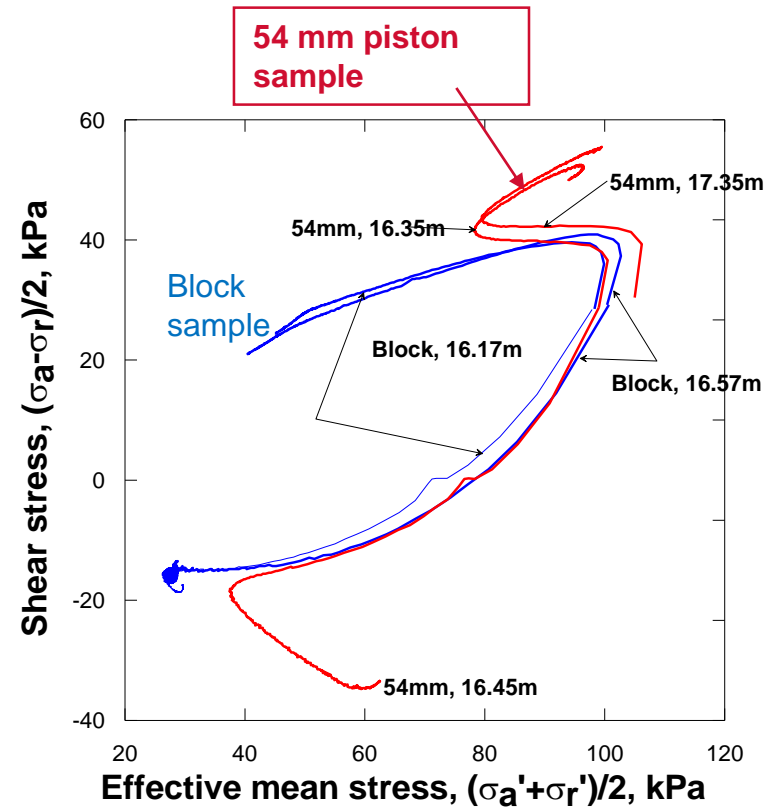
**Total cost savings of about USD 1.2 mill or 25 % of total
contract cost**

Low plasticity Norwegian clays ($I_p < 15\%$) are more susceptible to sample disturbance compared to high plasticity clays

DRAMMEN LOW PLASTICITY CLAY

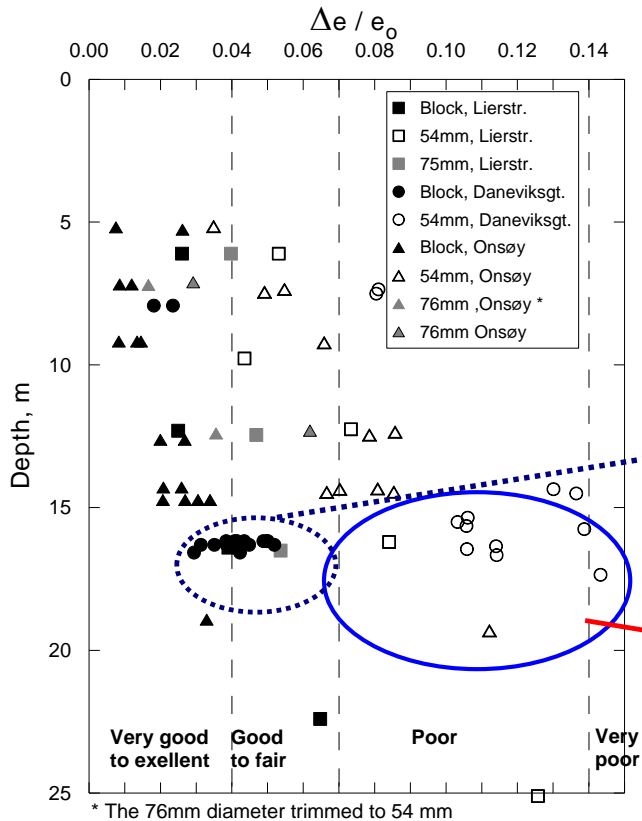


Stress – strain curve



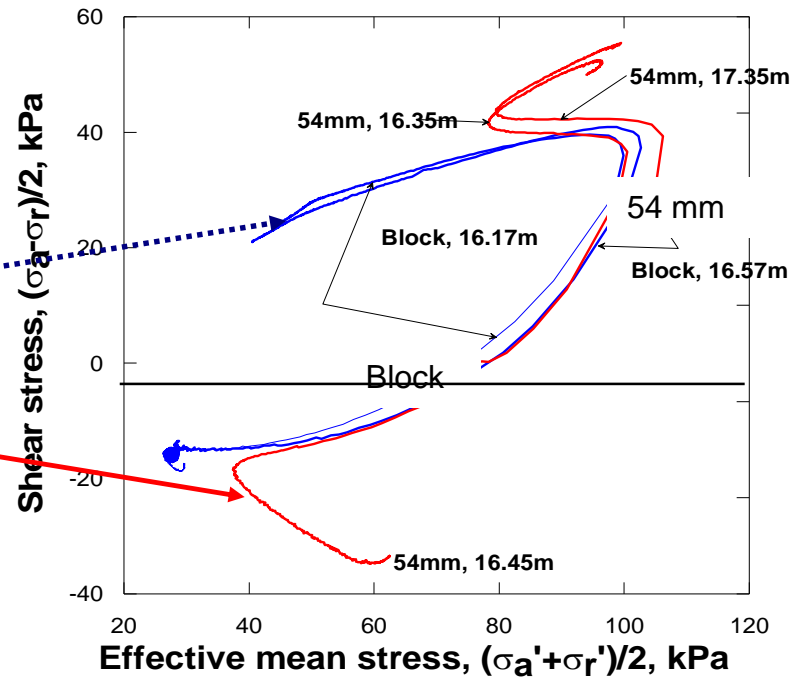
Stress path curve

Effect of sample disturbance is to change material behaviour from contractant to dilatant

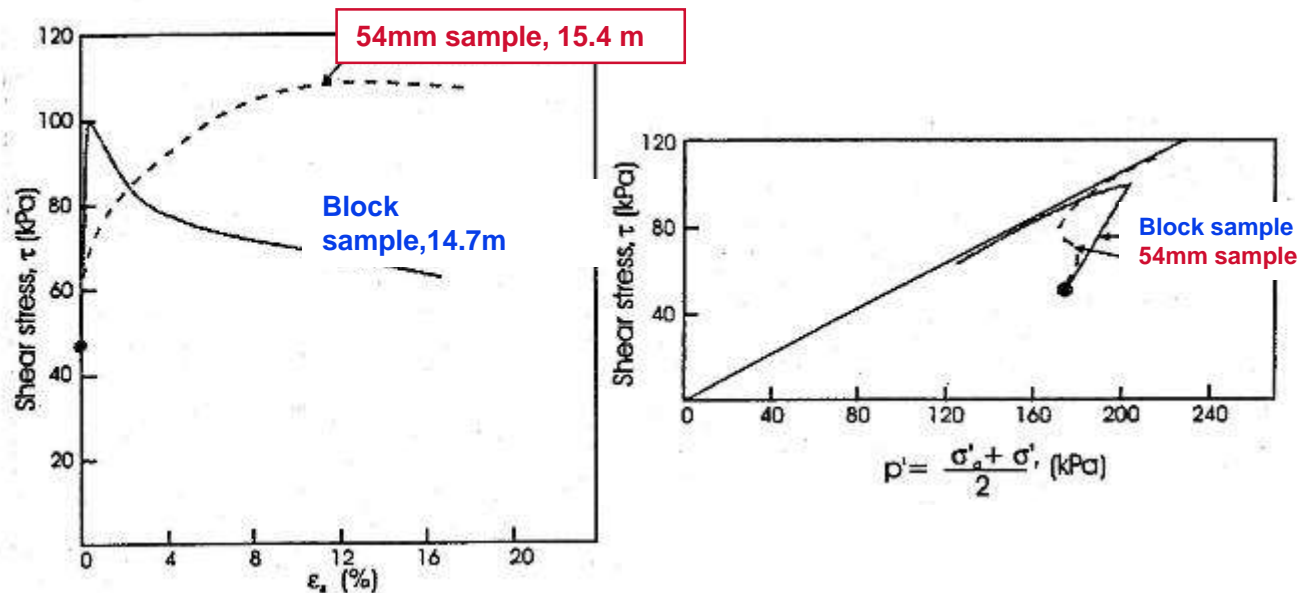


Low plasticity Drammen clay

Can be explained by large void ratio change $\Delta e / e_0$ when consolidating CAU specimen back to in situ stresses



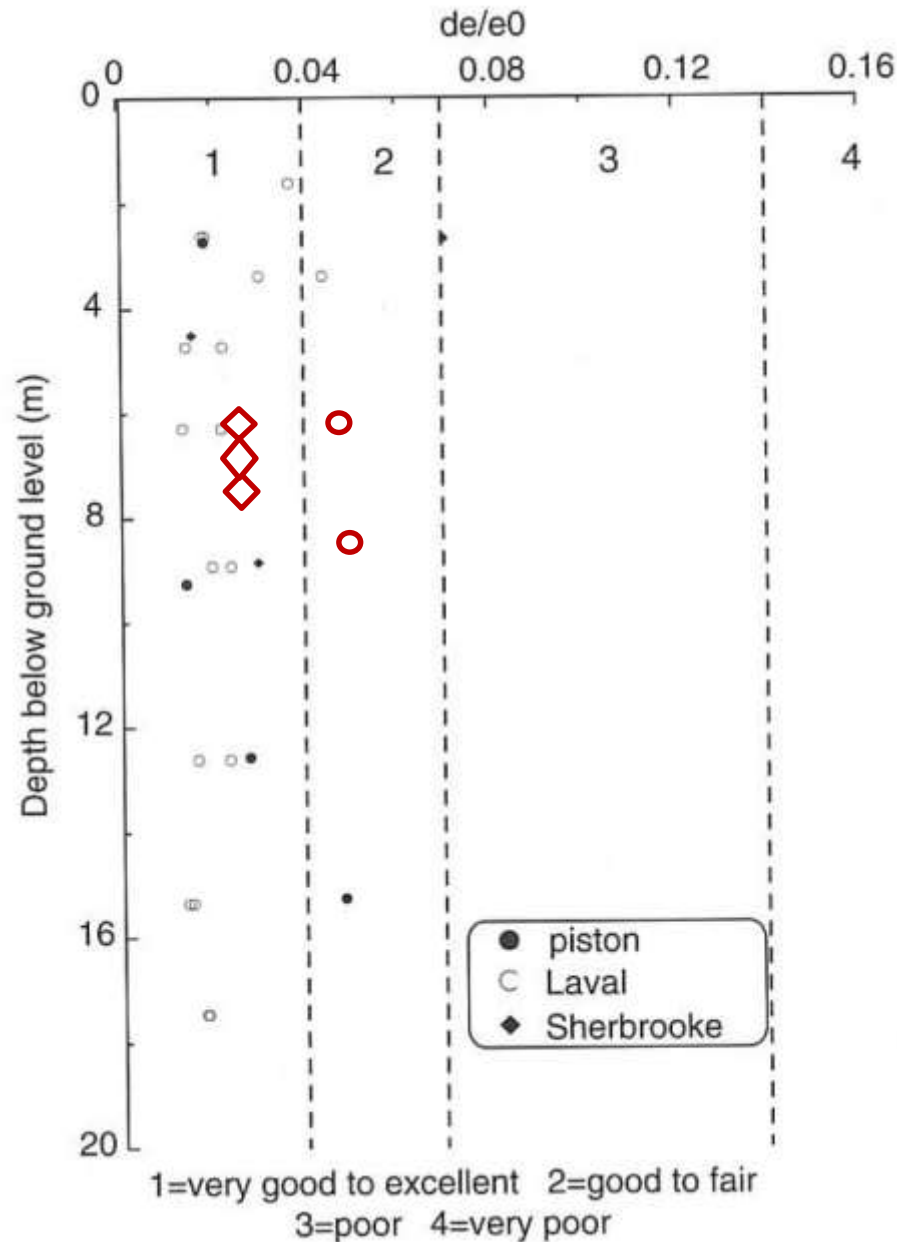
EIDSVOLD CLAY - CAU TESTS



For some low plasticity clays volume change during reconsolidation is so large that behaviour change from contractant to dilatant – selecting s_u at high strains will be non- conservative

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Bothkennar, Scotland, estuarine clay

Clay has some organic content and higher I_p , less susceptible to sample disturbance. In general SQ criteria work well

Data from Hight et al. (2003) and some tests from NGI (red)

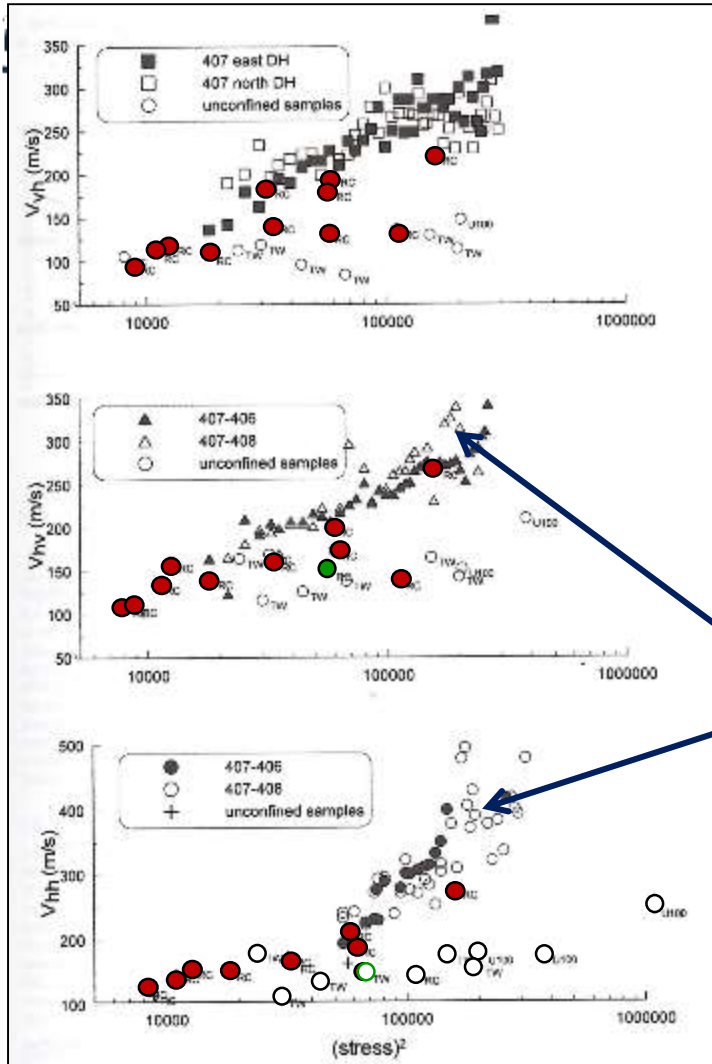
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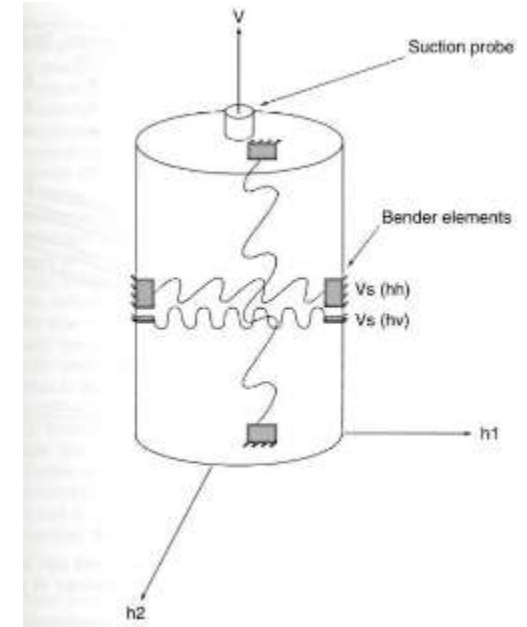
Need for other method(s) in overconsolidated clays

- Hight et al. (2003) have shown that in heavily OC clay tube sampling can induce negative pore pressure
- Can have tendency for swelling during consolidation
- In general it is recommended not to use NGI criteria for other than soft marine clay with $OCR < 4$

RESULTS OF SUCTION AND V_s MEASUREMENTS OC CLAY TERMINAL 5, HEATHROW



In situ measurements



Measurements on unconfined sample

- Tube sample
- Rotary core

SUMMARY AND CONCLUSION

- Based on requirements in offshore industry a sample quality (SQ) criteria has been developed for soft marine clays
- This has now been incorporated into new ISO standard for marine soil investigations and Norwegian adoption of Eurocode
- The criteria is now used in practise both offshore and onshore Norway
- The SQ criteria also works well for Bothkennar (UK) estuarine soft clay
- For heavily overconsolidated clays another approach is needed – measurements of shear wave velocity and suction as advocated by Prof. D. Hight is very promising